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# DEVELOPMENT DESIGN SPECIFICATION

# **D**8

# WATERFRONT DEVELOPMENT

## Amendment Record for this Specification Part

This Specification is Council's edition of the AUS-SPEC generic specification part and includes Council's primary amendments.

Details are provided below outlining the clauses amended from the Council edition of this AUS-SPEC Specification Part. The clause numbering and context of each clause are preserved. New clauses are added towards the rear of the specification part as special requirement clauses. Project specific additional script is shown in the specification as italic font.

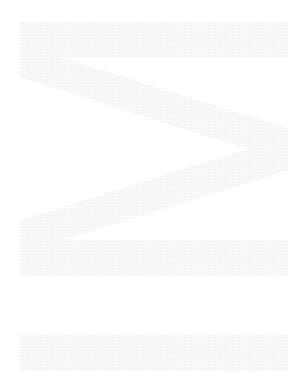
The amendment code indicated below is 'A' for additional script 'M' for modification to script and 'O' for omission of script. An additional code 'P' is included when the amendment is project specific.

Amendment Sequence No.	Key Topic addressed in amendment	Clause No.	Amendment Code	Author Initials	Amendment Date
1	Revised due to changes in state environmental planning policies	All	OAM	GTCC	June 05

# DESIGN SPECIFICATION D8 WATERFRONT DEVELOPMENT

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### DEVELOPMENT DESIGN SPECIFICATION D8 WATERFRONT DEVELOPMENT

### GENERAL

#### D8.01 SCOPE

1. The work to be executed under this Specification consists of the design of waterway facilities and structures for waterfront developments to meet the requirements of Council, and other relevant authorities.

2. This specification provides specific requirements related to developments that include water frontages to natural waterways and recreational facilities. The Specifications contained herein are to be considered supplementary to the requirements of Council's other design specifications.

3. Water front developments for the purposes of this specification shall be all developments within the coastal zone as specified in accordance with the Council Cadastral Mapping Series.

4. Canal estate developments, as defined by SEPP 50 Canal Development Section 3, shall not be undertaken, and as such are not covered within this Specification. Exceptions to this include development of any kind:

- a) that is required to complete a canal estate development in accordance with a development consent that was granted before the date of commencement of SEPP 50 and that has not lapsed, and
- b) that does not change the size or shape of the canals, or
- c) development that is needed for the maintenance of an existing canal.

#### D8.02 OBJECTIVE

1. The objective of this Specification is to provide both guidelines and requirements for Designers of developments that include water frontage. The requirements and guidelines seek to ensure waterfront development that is environmentally and structurally sound and minimises future maintenance and commitments.

#### D8.03 REFERENCE AND SOURCE DOCUMENTS

#### (a) Council Specifications

D1	-	Geometric Road Design
D3	-	Structures and Bridge Design
D5	-	Stormwater
D7	-	Erosion
DCP 36	-	Exempt and Complying Development

#### (b) Australian Standards

AS 2870.1	-	Residential slabs and footings - Construction
AS 3798	-	Guidelines on earthworks for commercial and residential
		developments.
AS 3962	-	Guidelines for Design of Marinas
AS 4678	-	Earth-Retaining Structures

Scope

Waterfront Developments - Definition

Canal Estate Developments AS 4997 - Design of Maratime Structures

#### (c) NSW State Legislation

Fisheries Management Act 1994 Protection of environment Administration Act 1991 Coastal Protection Act 1979 Environment Planning and Assessment Act 1979 Environment Planning and Assessment Regulation 2000 SEPP 71 Coastal Protection SEPP 14 Coastal Wetlands SEPP 50 Canal Development Water Management Act 2000 No 92

#### (d) State Authorities

PUBLIC WORKS DEPARTMENT, NSW.

- Floodplain Management Manual. (Jan 2001).
- Design Guidelines for Wharves and Jetties. Report No. 88062 (1990).
- Boat Launching Ramp Guidelines. Report No. PWD78024 (1985).
- Marina Guidelines. Report No. 87054 (1985).

#### (e) Bibliography

Australian Runoff Quality Manual, June 2003.

#### D8.04 CONSULTATION

1. Consultation with public authorities is necessarily more comprehensive in the case of waterfront developments. Design proposals shall not be considered by Council until all relevant approvals from public authorities have been obtained. Relevant public *Authority Approval* authorities include, but are not limited to:

- The Department of Public Works and Services, NSW (PWD)
- · NSW Fisheries
- · Waterways Authority
- National Parks and Wildlife Services (NPWS)
- The Department of Infrastructure Planning and Natural Resources (DIPNR)

#### D8.05 GENERAL REQUIREMENTS

1. There are general requirements pertinent to waterfront development which are **Flood Levels** applied by Council or other public authorities. These requirements include:

- No adverse effect to flood levels and flows in the area.
- No adverse effect to erosion or deposition conditions within the existing *Erosion* environment.
- Revetment walling is to be located within the property boundary.
   *Revetment Walls*
- Compliance with Water Management Act 2000 No 92

- Environmentally sensitive designs.
- Structural Stability to withstand flood, debris, buoyancy and live loadings.
- Maintenance of Waterway Facilities where no buffer zone is required Maintenance (see AusSpec D7 for buffer zone specifications).

#### D8.06 LAND RECLAMATION

1. A detailed foundation investigation shall be carried out by a qualified engineer in accordance with Aus-Spec DQS to determine the long term bearing capacity of the site. The investigation shall include the bearing capacity of the in-situ and fill components of the foundation. It shall predict the settlement of the finished surface through time (without structural loading). The foundation investigation shall specify any procedures or provisions to ensure that the foundation performance of the site will be suitable for the proposed types of site development in accordance with AS3798.

2. Before any allotments can be sold to the public, a certificate shall be issued by a qualified practising geotechnical engineer, certifying that the site has achieved the desired standard of performance and each site is to be classified in accordance with AS 2870.1. Geotechnical

3. The design of structural foundations should be carried out by a qualified engineer in accordance with Aus-Spec DQS to ensure compatibility with the inherent foundation **Design** properties of the proposed site.

#### WATERWAYS

#### D8.07 PLANNING CONCEPTS

1. Waterway frontages shall be natural in appearance. Buffer zones, as specified in Aus-Spec D7, shall be used to ensure this. Designs for maintenance of existing canal structures shall also be kept natural in appearance and undertaken in consultation with Council.

2. Developments fronting existing waterways shall also ensure depths for **Moorings** navigation and mooring are available in accordance with the following:

- draught of boat
- underkeep clearance (UKC)
- allowance for sedimentation.

- 3. Water quality must be such that the following are not adversely affected:
  - occasional swimming and wading
  - boating
  - passive recreation
  - visual aesthetic acceptability
  - freedom from excessive plant and algal growth
  - the maintenance of a complete aquatic faunal community.

#### D8.08 RESERVED

#### D8.09 WATERWAY DEPTHS

1. Depths should be sufficient for safe navigation by craft likely to use the waterway, except in non navigable areas which may be set aside for creation of wetland habitats.

2. Waterway depths for existing waterways should be sufficient for safe navigation by craft likely to use the waterway and a minimum of 2m is preferred.

#### D8.10 WATERWAY WIDTHS

1.Two measurements for width can be distinguished:Width<br/>Definition

- Navigation width: width at the navigation depth (natural waterway and existing canals)
- Overall width:
  - \* width between the top of the revetment walls (existing canals); and
  - \* with from bank to bank (natural waterway)

These two widths are shown in Figure D8-2 for a typical existing channel "Dry Beach" cross section.

2. The navigation widths for Waterways shall be sufficient for safe navigation by two-way and one-way boat traffic respectively, taking into account the size of craft likely to use the waterway. Minimum navigation widths shall be 5 x Bmax or 20m whichever is the greater.

Where Bmax is the maximum beam of the craft likely to use the waterway.

Where any structures or moored craft encroach into the navigation width, a clear distance of 5 x Bmax shall be provided measured between structures or craft moored on opposite sides of the waterway.

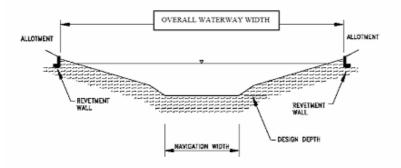
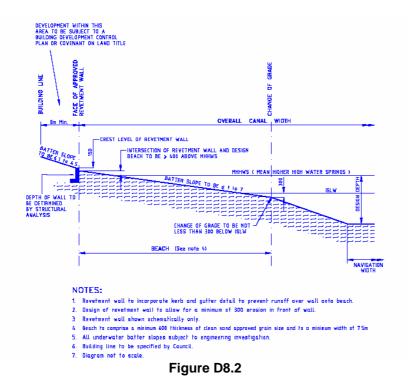


Figure D8.1 Typical Existing Channel Beach Cross Section

3. width of less	Craft shall not be permitted to navigate into waterways with a navigation s than 25m.	Navigation Width
4.	The overall width shall be determined by appropriate hydrological studies.	Minimum Overall Width
5. bank and be	Determination of the navigation and overall widths shall take into account ed stability considerations.	
D8.11	RESERVED	
D8.12	WATERWAY FRONTAGE PROFILES	
DQS, taking including lo assessmen	The cross-section and edge treatment shall be designed in accordance with neering practice by a qualified civil engineer in accordance with Aus-Spec g into account the type of soil conditions, the likely range of water levels ng term variations, and the applied forces, including wind and wave ts. Engineering studies demonstrating the adequacy of the cross-sections eatment shall be made available to the DIPNR.	Engineering Studies
however, al rock will be	The cross-section design for existing land developments adopting a beach ld conform in principle with the design cross-sections shown in Figure D8.2 ternative designs such as structural retaining walls, rock gabions and mass considered where necessary for maintenance works and only after with Council.	Alternatives



#### No Beach Submerged Cross Section – Existing Canals Only

#### D8.13 UNDERWATER BATTERS AT WATERWAYS AND SHORELINES

1.

The typical ranges of stable underwater batters shall be determined by a Geotechnical report and structural analysis. Table D8-1 has been included for guidance only.

#### Table D8-1

#### Effect of Material Type On Underwater Batter – For Guidance Only

Material	Stab	le Underwater Ba	atter
Stiff Clay		1 : 1.5	
Firm Clay	1:2	to	1 : 4.5
Sandy Clay	1:4	to	1:7
Course Sand	1:3	to	1:6
Fine Sand	1:5	То	1 : 10
Mud	1:8	to	1 : 50

2. Stormwater outlets into beach type waterways are to be designed after Consultation with Council. They are preferred to be above mean high watermark. If this cannot be achieved, after consultation with Council, a headwall and extended apron shall Stormwater Outlets be provided for access for maintenance operations.

3. Use of course granular materials > 20 mm for beach formation, eg gravels and cobbles, may cause problems in the form of oyster and barnacle growth. A well graded soil up to 20 mm in diameter shall be used.

#### D8.14 ENTRANCES TO MARINAS & HARBOURS

1. sufficient	For navigable entrances, the width and depth of the entrance shall be t for safe navigation by craft likely to use the waterway.	Navigation
2.	Refer to D8.07 point 3 "Waterways" for depth selection factors.	Depth Selection
3.	Extensive modelling may be required to account for tidal influences, see	Modelling

3. Extensive modelling may be required to account for tidal influences, see **Wodelling** Section D8.17 Note 2.

## HYDRAULICS

#### D8.15 STORMWATER MANAGEMENT

1. The proposed development shall not adversely affect water quality within the development and host waterbody, or lead to problems associated with siltation and erosion. Design concepts and details for site specific stormwater treatments shall ensure:

a)	The development allotment shall be graded to ensure as much runoff as
	possible is directed to the street where it may be collected and then directed
	into the waterway through properly designed stormwater outlets.

- b) Wherever practical, stormwater outlets shall be located at points of maximal flushing, or directly with the host waterbody. **Flushing**
- c) Sediment control shall be in accordance with Aus-Spec D5 and D7.

#### D8.16 FLOOD CONTROL STRUCTURES

1. Flood control structures include devices such as: weirs, flow deflectors, and rock **Definition** protection.

2. Flood control structures are to be approved by the DIPNR and Fisheries. Usually detailed designs for flood control structures are commenced only after the overall waterway system and flood structures have been mathematically and physically modelled and approved by the DIPNR and Council. Preliminary plans are usually prepared as part of a "flood study" which involves modelling procedures.

3. Designs must ensure that the proposed works and any raising of the land will not result in any significant increase in flood levels in the area.

#### D8.17 TIDAL INFLUENCES

1. The level of study of tidal hydraulics necessary to adequately demonstrate the impact of the proposed development on tidal hydraulics, and the effects of the tidal

Water Quality

Control

hydraulics on the development, is dependent on a number of factors. It is important that the proponent seek early consultation with the DIPNR.

2. It is likely that detailed studies, involving mathematical and/or physical modelling, will be necessary where it is proposed that the development have more than one entrance, where an understanding of internal tidal circulations is important, or where the development is located within the entrance reach of the host estuary and would involve significant changes to the frictional and shallow water controls on tidal propagation.

3. The Designer shall assess variations in the tidal characteristics of the host estuary at the development site taking into account cyclic and long term changes in: estuary shoaling and scour, entrance stability, hydrologic input, mean sea level, and any engineering works (such as large scale estuary dredging, entrance works, or other canal subdivisions) proposed or approved by the various government authorities. The implications of these changes to the design and functionality of the development shall be established and accommodated.

4. The Designer shall establish the tidal levels at the proposed development site. **Tidal Levels** These levels may be based on existing information supplied by the DIPNR, where available, or measurements undertaken on behalf of the proponent by a suitably qualified surveyor or civil engineer.

5. There is no minimum acceptable tidal range below which developments would not be considered. The degree of tidal flushing will however reduce as tidal range decreases, and this effect must be considered in the water exchange and mixing studies required by the Department and outlined elsewhere in the guidelines.

6. Flood control structures, such as weirs, significant rock protection and groynes are to be designed and certified by an Engineer in accordance with Aus-Spec DQS. As part of the design certification the designer shall provide to Council evidence of issue of the relevant licenses issued under the Water Management Act 2000, and the Fisheries Management Act and PDE Act. All designs shall be undertaken in accordance with the Australian Standards and Statutory Authority Guidelines relevant to each element of the design.

#### D8.18 RESERVED

#### D8.19 EROSION AND SEDIMENTATION INFLUENCES

1. Sandy beaches within developments require maintenance (nourishment) at regular intervals. Where recovery of the eroded sand from the bed of the waterway is unlikely to be feasible, it is necessary to make allowance for ongoing sedimentation on the waterway bed from this source.

2. Shoreline structures which extend across the littoral drift zone, eg. some **Drift** stormwater outlet designs, should be avoided where relatively high littoral drift rates are anticipated, except where special provision has been made to mitigate beach erosion.

3. The sediment load carried by stormwater outlets from a given catchment area **Sediment Load** shall be determined in accordance with Aus-Spec D7.

#### STRUCTURES

#### D8.20 REVETMENT WALLS FOR EXISTING CANAL DEVELOPMENTS ONLY

1. Revetment walls are to be designed in accordance with Aus-Spec D3.

2. Filling is to be composed of material not injurious to the health of the neighbourhood and shall comply with Council's requirements for filling in subdivisions *Walls* 

3. The crest of the revetment wall above the design profile, for the particular type of waterway cross-section adopted, shall conform with the requirements set out in Table D8-2.

4. There is no maximum height for revetment walls as such. However, consistent with the requirements in Table D8-2 the crest level of revetment walls should be kept as low as possible to enable easy access from the allotments onto the waterway, to optimise mixing wind action, and to reduce visual impact.

5. Determination of the full construction height, structural adequacy and stability of the wall shall take into account erosion allowance and subsoil drainage facilities, and hydraulic processes, in front of the wall. In the absence of detailed hydraulic tests the allowances for erosion shall not be less than the values specified in Table D8-3.

#### **CANAL CROSS-SECTION HEIGHT OF REVETMENT WALL ABOVE DESIGN** WATERWAY PROFILE Dry Beach Minimum of 150mm Inter-Tidal Beach Sufficiently high to accommodate MHHWS\* plus wind setup, wave runup, long term changes in mean sea level and local tide levels, without overtopping. No Beach - Submerged Sufficiently high to accommodate MHHWS plus Slope wind setup, wave runup, long term changes in mean sea level and local tide levels, without overtopping. No Beach - Vertical Wall Sufficiently high to accommodate design water depth, MHHWS, wind setup, wave runup, long term changes in mean sea level and local tide levels, without overtopping.

# TABLE D8-2FACTORS FOR DETERMINING HEIGHT OF REVETMENT WALLS

(\* MHHWS = Mean Higher High Water Springs)

# TABLE D8-3 MINIMUM EROSION ALLOWANCES FOR REVETMENT WALLS

CANAL CROSS-SECTION	MINIMUM EROSION ALLOWANCE IN FRONT OF REVETMENT WALL (MM)
Dry Beach	300
Inter-Tidal Beach	450

No Beach - Submerged Slope	300
No Beach - Vertical Wall	300

7. To mitigate against beach erosion, runoff from the slope behind the revetment wall should be interrupted and dissipated by infiltration or broad sheet flow prior to flowing over the revetment wall onto the beach.

#### D8.21 JETTIES, PONTOONS AND BOAT RAMPS

1. Where jetties and pontoons are proposed for waterways which serve as floodways, the effect of these structures on the hydraulic performance of the waterways *Jetties* shall be taken into account in the hydraulic design of the waterways.

2. Jetties, pontoons and boat ramps shall be designed in accordance with sound engineering practice by a qualified Civil Engineer, in accordance with Aus-Spec DQS, to satisfactorily resist all dead loads and applied live loads. Jetties, pontoons and boat ramps shall be designed to meet the requirements of DCP 36 Part A 'Jetties and Boating Structures'. Particular consideration shall be given to the effect of flood currents and debris loading on structures proposed to be located within waterways which will serve as floodways.

3. Special design requirements due to the height of water levels during flooding shall also be considered, eg. Electrical connections and cut-off levels for mooring piles. *Electrical Connections* 

4. Account shall be taken of jetty pontoon, ramp, etc design in assessing the required width of the waterways.

4. Where a hinged access ramp leads to a pontoon the slope of the hinged access ramp shall not exceed 1 in 6 at the lowest anticipated water level. Where pontoons are to be provided, fixed-jetties can be used to reduce the length of hinged access ramps but shall not extend past the revetment wall by a distance greater than 7m. The level of the jetty deck shall not be greater than 300mm above the top of the revetment wall, and the deck shall not rest on the wall. The overall length from the revetment wall to the outer edge of the mooring structure shall not exceed 17m and shall not extend into the navigation channel.

5. Boat ramps for individual allotments are acceptable only in the Dry Beach and Inter-Tidal Beach cross-sections. They shall be constructed of concrete and be not less than 150mm thick on the canal side of the revetment wall and have a width not less than 3m. Isolation joints are to be provided so that the concrete slabs forming the ramp are not supported by the revetment wall and can move independently of the wall.

7. The ramp shall not extend below the position of the change in grade at ISLW- **End of Ramp** 0.3m (Canal Cross-Section/Edge Treatment: Figure D8.2).

8. A boat ramp can be constructed with its surface either flush with beach surface or the top of the revetment wall at the point of intersection. There are advantages and disadvantages with each approach which should be evaluated during the determination of a standard design. The following issues should be considered:-

- Boat ramps flush with waterway beach:
  - structural design of the revetment wall will need to allow for recessing
  - allotment surface drainage control could be disrupted.
- Boat ramps flush with top of revetment wall:
  - ramp will be proud of beach profile which could lead to beach scour through groyne action and local wave reflections

ramp will constitute an impediment to access for maintenance vehicles (if required).

9. Boat ramps and other structures should be designed to minimise their visual **Visual Impact** impact. Boat ramps having their surfaces level with the canal beach surface are less visually prominent and are therefore preferable, in terms of visual impact, to ramps which project above the beach surface.

10. Adequate provision shall be made to ensure that scour does not occur under any part of the ramp, eg. by founding the ramp on stable, non-erodible, material and/or incorporating deeper edge beams.

11. Reference should also be made to the "Boat Launching Ramp Guidelines" **Other** (Public Works Department, 1985) and "Design Guidelines for Wharves and Jetties" **References** (Public Works Department, 1990).

12. A standard design could be considered for jetties, pontoons and boat ramps (including the means of shore connection) that are proposed as part of the development *Standard Designs* or that may be constructed by owners at a later date.

13. Public boat launching facilities and marina facilities shall be developed in *Public* accordance with the Public Works Department's "Boat Launching Ramp Guidelines" and *Facilities* "Marina Guidelines" respectively.

14. Where it is proposed to construct public boat launching facilities within a canal development, consideration shall be given to siting of the launching facilities so as to minimise any adverse noise impacts on adjacent development due to the particular hours of use of the facilities.

15. At public boat launching ramps, consideration should be given to providing holding beaches and boarding jetties or pontoons to facilitate rigging and efficient boarding of craft. *Holding Beaches at Ramps* 

16. Disables access shall be considered in design of these conditions.

#### D8.22 BRIDGES AND STRUCTURES

1. Bridges and structures shall be designed in accordance with the Specification for Structures and Bridge Design (D3).

2. The vertical clearance of any proposed bridge should be checked with the Waterways Authority.

## SPECIAL REQUIREMENTS

#### D8.23 CERTIFICATION

The designer shall be required to certify the design and ultimately certify the WAE plans for compliance with design in accordance with Aus-Spec DQS and Aus-Spec D14 WAE Plans.

#### D8.24 RIPARIAN VEGETATION

Developments with frontage to existing waterways shall have a buffer. The buffer shall be determined in accordance with Aus-SpecD5 and D7.

D8.25 RESERVED